

ABBREVIATIONS

A.B.	ANCHOR BOLT
ADDL.	ADDITIONAL
ALT.	ALTERNATE
ARCH.	ARCHITECTURAL
B.	BOTTOM
BLKG.	BLOCKING
BM.	BEAM
B.N.	BOUNDARY NAIL
B.O.F.	BOTTOM OF FOOTING
BRG.	BEARING
BTN.	BETWEEN
BOT.	BOTTOM
CBC.	CALIFORNIA BUILDING CODE
C.G.	CENTER OF GRAVITY
C.I.P.	CAST-IN-PLACE
CL.	CENTER LINE
CLR.	CLEAR
COL.	COLUMN
CONC.	CONCRETE
CONN.	CONNECTION
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
DBL.	DOUBLE
DBA.	DOWEL BAR ANCHOR
DL.	DEAD LOAD
DP.	DEEP
DIA.	DIAMETER
DIM.	DIMENSION
DO.	DITTO (REPEAT)
DWG.	DRAWING
DWL.	DOWEL
(E)	EXISTING
EA.	EACH
E.F.	EACH FACE
EL.	ELEVATION
ELEC.	ELECTRICAL
EMBED.	EMBEDMENT
EQ.	EQUAL OR EQUIVALENT
E.S.	EACH SIDE
E.W.	EACH WAY
EXT.	EXTERIOR
FDN.	FOUNDATION
F.F.	FINISHED FLOOR
F.O.C.	FACE OF CONCRETE
F.O.M.	FACE OF MASONRY
F.S.	FAR SIDE
FTG.	FOOTING
GA.	GAUGE
GALV.	GALVANIZED
GR.	GRADE
H.	HORIZONTAL
HT.	HEIGHT
HORIZ.	HORIZONTAL
H.S.H.	HORIZONTALLY SLOTTED HOLES
I.D.	INSIDE DIAMETER
I.F.	INSIDE FACE
INT.	INTERIOR
I.O.R.	INSPECTOR OF RECORD
JT.	JOINT
LDGR.	LEDGER
LL.	LIVE LOAD
LT. WT.	LIGHT WEIGHT
MATL.	MATERIAL
MAX.	MAXIMUM
M.B.	MACHINE BOLT
MECH.	MECHANICAL
MFR.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
MTL.	METAL
N.I.C.	NOT IN CONTRACT
NO.	NUMBER
N.S.	NEAR SIDE
N.T.S.	NOT TO SCALE
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
O.F.	OUTSIDE FACE
O.H.	OPPOSITE HAND
OPNG.	OPENING
OPP.	OPPOSITE
ORIG.	ORIGINAL
PL.	PLATE
PLCS.	PLACES
P.W.	PLATE WASHER
R.	RADIUS
REF.	REFERENCE
REINF.	REINFORCEMENT
REQD.	REQUIRED
R.W.	RETAINING WALL
SCHED.	SCHEDULE
SECT.	SECTION
SEOR.	STRUCTURAL ENGINEER OF RECORD
SEP.	SEPARATION
SIM.	SIMILAR
S.P.	SEE PLAN
SPEC.	SPECIFICATION
SQ.	SQUARE
S.S.	STAINLESS STEEL
STD.	STANDARD
STGR.	STAGGER
STIFF.	STIFFENERS
STR.	STRUCTURAL
STR. I	STRUCTURAL I
T.	TOP
T&B	TOP & BOTTOM
THK.	THICKNESS/THICK
THRD.	THREADED
T.O.P.	TOP OF PARAPET
T.O.S.	TOP OF SLAB
T.O.W.	TOP OF WALL
Typ.	TYPICAL
UBC.	UNIFORM BUILDING CODE
U.N.O.	UNLESS NOTED OTHERWISE
V.	VERTICAL
VERT.	VERTICAL
V.S.H.	VERTICAL SLOTTED HOLES
W.P.	WATERPROOF OR WORK POINT
W.P.J.	WEAKENED PLANE JOINT
WT.	WEIGHT
W.W.F.	WELDED WIRE FABRIC
W/	WITH
W/O	WITHOUT
W.O.	WHERE OCCURS
XS	EXTRA STRONG (STEEL PIPE)
XXS	DOUBLE EXTRA STRONG (STEEL PIPE)

DSA GENERAL STRUCTURAL NOTES:

GENERAL:

THE STRUCTURAL CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE STRUCTURE IS DESIGNED TO BE A STABLE UNIT AS A COMPLETED WHOLE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DESIGN, ERECT AND INSPECT TEMPORARY SHORES, BRACES, ETC. TO SUPPORT THE STRUCTURE AGAINST ALL ANTICIPATED LOADS INCLUDING GRAVITY, WIND AND LATERAL EARTH PRESSURE UNTIL ITS COMPLETION. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OF THESE METHODS OF CONSTRUCTION. CONSTRUCTION MATERIAL SHALL BE PLACED ON FRAMED FLOORS AND ROOFS SUCH THAT THE DESIGN LIVE LOADS ARE NOT EXCEEDED.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS AND SITE CONDITIONS PRIOR TO STARTING CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT.

WORKMANSHIP AND MATERIALS SHALL COMPLY WITH THE LATEST EDITIONS OF THE CALIFORNIA BUILDING CODE AND ALL "A" CHAPTER AMENDMENTS AND TESTING STANDARDS.

NOTES AND DETAILS ON THE DRAWINGS TAKE PRECEDENCE OVER THE GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT. "TYPICAL" DETAILS ARE NOT FLAGGED ON THE DRAWINGS, BUT APPLY UNLESS NOTED OTHERWISE.

THESE DRAWINGS REPRESENT WORK TO BE PERFORMED IN THE AREA OF SCOPE ONLY. THE ENGINEER TAKES NO RESPONSIBILITY FOR ANY OTHER PORTIONS OF THIS STRUCTURE.

ALL NEW DESIGN PER CBC 2022 AND ALL "A" CHAPTER AMENDMENTS.

ALL EXISTING CONDITIONS ASSUMED IN DETAILS MUST BE CONFIRMED IN THE FIELD PRIOR TO FABRICATION OR CONSTRUCTION. ANY DISCREPANCIES MUST BE BROUGHT TO THE ATTENTION OF COFFMAN ENGINEERS BEFORE CONSTRUCTION OR FABRICATION.

COORDINATION:

ALL DRAWINGS ARE CONSIDERED TO BE PART OF THE CONTRACT DOCUMENTS. ANY DISCREPANCIES THAT ARE FOUND SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO START OF CONSTRUCTION. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT HIS OWN EXPENSE AND AT NO EXPENSE TO THE OWNER OR ARCHITECT.

ALL DEPRESSIONS, ELEVATIONS AND SLOPES SHOULD BE COORDINATED WITH THE ARCHITECT AND PLANNED FINISHES. ALL WEATHER EXPOSED SURFACES SHOULD BE SLOPED TO DRAIN PER ARCHITECTURAL OR OTHER DRAWINGS.

COORDINATION SHALL INCLUDE, BUT NOT BE LIMITED TO, DETERMINING THE LOCATION OF ALL EXISTING UTILITY LINES THAT MAY INTERFERE WITH THE INSTALLATION OF THE NEW STRUCTURAL ELEMENTS. THE CONTRACTOR SHALL INFORM THE ARCHITECT IF INTERFERENCE IS ENCOUNTERED. THE ARCHITECT WILL DETERMINE IF THE LINES ARE TO BE REMOVED OR RELOCATED.

UNLESS OTHERWISE NOTED ON THE DRAWINGS, DO NOT PENETRATE ANY STRUCTURAL ELEMENTS SUCH AS BEAMS, COLUMNS, WALLS, SLABS, ETC. WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.

SHOP DRAWINGS AND SUBMITTALS:

SHOP DRAWINGS AND SUBMITTALS TO THE ENGINEER ARE REQUIRED FOR ALL STRUCTURAL WORK, UNLESS OTHERWISE AGREED TO BY THE ARCHITECT/ENGINEER. NO WORK SHALL PROCEED WITHOUT THE REVIEW AND APPROVAL OF SHOP/RECTIONAL LAYOUT DRAWINGS BY THE ARCHITECT AND ENGINEER. ALL SUBMITTALS SHALL BE MADE IN A TIMELY MANNER ALLOWING ADEQUATE TIME AS NEEDED BY THE ARCHITECT AND ENGINEER FOR REVIEW.

THE CONTRACTOR SHALL REVIEW AND APPROVE ALL SHOP DRAWINGS PRIOR TO ENGINEERING REVIEW. SUBMISSIONS FOR ENGINEERING REVIEW SHALL INCLUDE A REPRODUCIBLE AND ONE COPY, REPRODUCIBLE WILL BE MARKED AND RETURNED. ELECTRONIC SUBMITTALS MAY BE ACCEPTED.

ANY STRUCTURAL ENGINEERING DESIGN PROVIDED BY OTHERS SHALL BE SUBMITTED FOR REVIEW AND SHALL BEAR THE SEAL OF A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.

TRADES PROVIDING SHOP DRAWINGS SHOULD PROPERLY COORDINATE WITH ALL OTHER ALL OTHER TRADES AND SHOW OTHER POSSIBLE CONFLICTS OR OVERLAPPING ELEMENTS FROM OTHER TRADES ON THEIR SUBMITTAL FOR REVIEW.

REQUIRED SHOP DRAWINGS AND SUBMITTALS:

STRUCTURAL STEEL SHOP DRAWINGS AND MILL CERTS
CONCRETE REINFORCEMENT SHOP DRAWINGS AND MILL CERTS
ANCHOR BOLT LAYOUT AND MATERIAL SPECS
CONCRETE MIX DESIGNS AND GROUT PRODUCTS

INSPECTIONS AND SPECIAL INSPECTIONS:

THE OWNER SHALL EMPLOY AN INSPECTOR TO PROVIDE INSPECTION PER CBC 2022 AND ALL "A" CHAPTER AMENDMENTS. ALL SPECIAL INSPECTORS SHALL BE APPROVED BY THE DIVISION OF STATE ARCHITECT (DSA).

STRUCTURAL OBSERVATIONS:

STRUCTURAL OBSERVATION IS REQUIRED FOR THE STRUCTURAL SYSTEM IN ACCORDANCE WITH CBC CHAPTER 17A. STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE ELEMENTS AND CONNECTIONS OF THE STRUCTURAL SYSTEMS AT SIGNIFICANT CONSTRUCTION STAGES AND THE COMPLETED STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS. STRUCTURAL OBSERVATION DOES NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED OF THE BUILDING INSPECTOR OR THE DEPUTY INSPECTOR. STRUCTURAL OBSERVATIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF CAC CHAPTER 7 AND CBC CHAPTER 17A.

THE STRUCTURAL OBSERVER SHALL PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL OBSERVER.

CONSTRUCTION STAGES AND ELEMENTS TO BE OBSERVED

PRIOR TO CONCRETE PLACEMENT/FOUNDATION REINFORCEMENT AND ANCHOR BOLTS
PRIOR TO COVERING WALLS AND FRAMING

CODE:

2022 CALIFORNIA BUILDING CODE AND ALL "A" CHAPTER AMENDMENTS.

SEISMIC:	
SEISMIC RISK CATEGORY	III
SITE CLASS	C
IMPORTANCE FACTOR	1.25
SEISMIC DESIGN CATEGORY	D
Ss	2.007
S1	0.716
S0s	1.606
S01	0.668
SEISMIC FORCE RESISTING SYSTEM	SPECIAL CANTILEVERED COLUMNS
R	2.5
Cs	0.803

SNOW LOADS DO NOT APPLY TO THIS SITE.
FLOOD LOADS DO NOT APPLY TO THIS SITE.
WIND LOADS DO NOT APPLY TO THIS SITE.

FOUNDATION:

GEOTECHNICAL REPORT BY: NINYO & MOORE
475 GODDARD, SUITE 200
IRVINE, CA 92618
PHONE: (949) 753-7070
DATE: APRIL 11, 2024 PROJECT NO: 211415039

GEOTECH RECOMMENDATIONS:
NET ALLOWABLE BEARING CAPACITY FOR PIER FOOTING

	= 3000 PSF
	(24" WIDE MIN., 24" DEEP MIN., 200 PSF INCREASE PER FT. WIDTH & 400 PSF INCREASE PER FT. FOR DEPTH INCREASE BEYOND MINIMUM, 4000 PSF MAX.)
DEPTH	= 24" MIN. DEEP
Pp	= 350 PCF (3500 PSF MAX.)
	(1/3 INCREASE FOR WIND OR SEISMIC)
μ	= 0.4

SOIL ENGINEER TO INSPECT AND APPROVE FOUNDATION AND SUBMIT APPROVAL TO DSA, PRIOR TO DSA INSPECTION.

SOIL BENEATH BOTTOM OF FOOTINGS AND SLAB SUB-BASE TO BE VERIFIED IN WRITING BY SOILS ENGINEER AS MEETING THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. IF ADVERSE SOIL CONDITIONS ARE ENCOUNTERED DURING CONSTRUCTION, CONSULT THE GEOTECH.

DEFERRED APPROVALS:

ANY STRUCTURAL ENGINEERING DESIGN PROVIDED BY OTHERS SHALL BE SUBMITTED FOR REVIEW AND SHALL BEAR THE SEAL OF A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED. THE APPROVAL SHALL BE REVIEWED BY THE CONTRACTOR, SEOR, AOR, AND DSA.

DEFERRED APPROVALS:

SUPPORTS, ANCHORAGE, AND BRACING OF ALL DISTRIBUTION SYSTEMS INCLUDING BUT NOT LIMITED TO PIPING, DUCTWORK, CONDUIT, PNEUMATIC TUBE, FIRE SPRINKLERS, AND INLINE COMPONENTS SUCH AS VAV AND CAV AND FILTER BOXES NOT SPECIFICALLY DETAILED ON STRUCTURAL DRAWINGS.

CONCRETE:

CONCRETE CONSTRUCTION SHALL CONFORM WITH THE LATEST EDITION OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" AND ACI 318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". SUBMIT MIX DESIGNS FOR EACH CLASS OF CONCRETE. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE UNLESS NOTED OTHERWISE. ALL LIGHTWEIGHT CONCRETE TO BE SAND LIGHTWEIGHT CONCRETE.

CONCRETE CONTAINING SUPERPLASTICIZING ADMIXTURE SHALL HAVE A SLUMP NOT EXCEEDING 3", TO BE FIELD VERIFIED, PRIOR TO ADDING ADMIXTURE, AND NOT EXCEEDING 9" AT PLACEMENT. CONCRETE NOT CONTAINING SUPERPLASTICIZING ADMIXTURE SHALL HAVE A SLUMP NOT EXCEEDING 9" AT PLACEMENT. ADDITION OF WATER TO A MIX WITH INSUFFICIENT SLUMP WILL NOT BE PERMITTED, EXCEPT AS ALLOWED PER ASTM C494.

MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED, EXCEPT THAT SLABS ON GRADE NEED BE VIBRATED ONLY AROUND UNDER-FLOOR DUCTS, ETC. CAST CLOSURE POUR AROUND COLUMNS AFTER DEAD LOAD IS APPLIED.

MINIMUM CONCRETE MIX DESIGN REQUIREMENTS SHALL BE AS FOLLOWS:

ITEM	28 DAY STRENGTH F'c (PSI)	MAX. SIZE AGGREGATE	DENSITY
FOUNDATIONS	3,000	1"	150 PCF
SLAB ON GRADE	3,000	3/4"	150 PCF

NOTE: A HIGHER GRADE OF CONCRETE MAY BE SUBSTITUTED FOR THOSE SHOWN ABOVE BUT WILL BE SUBJECT TO THE CODE REQUIREMENTS OF THE HIGHER GRADE.

AGGREGATE AND AGGREGATE GRADATION SHALL CONFORM TO ASTM C33.

AIR ENTRAINMENT IS OPTIONAL AND WILL BE REVIEWED IN MIX SUBMITTAL.

FLY ASH SHALL BE CLASS F ONLY AND IS OPTIONAL AND WILL BE REVIEWED IN MIX SUBMITTAL.

WET CURING IS RECOMMENDED SUBMIT METHOD FOR CURING TO SEOR.

CONCRETE COVER OVER REINFORCING SHALL BE AS FOLLOWS U.N.O.:

POURED AGAINST EARTH	3"
EXPOSED TO EARTH, BUT POURED AGAINST FORMS	2"
TYPICAL BARS IN COLUMNS WALLS AND BEAMS	1 1/2"
RAISED STRUCTURAL SLAB	3/4"

BEFORE CONCRETE IS POURED, CHECK WITH ALL TRADES TO INSURE PROPER PLACEMENT OF ALL OPENINGS, SLEEVES, CURBS, CONDUITS, BOLTS, INSERTS, ETC. RELATING TO WORK.

ALL SLEEVES NOT SPECIFICALLY SHOWN ON DRAWINGS SHALL BE LOCATED BY THE TRADES INVOLVED & SHALL BE APPROVED BY STRUCTURAL ENGINEER.

DRYPACK CONCRETE SHALL BE ONE PART PORTLAND CEMENT & ONE PART SAND WITH SUFFICIENT WATER TO ALLOW A SMALL AMOUNT OF PASTE TO COME TO THE SURFACE.

CONCRETE GROUT SHALL BE NON-SHRINKING WITH SUFFICIENT WATER TO ALLOW POURING. MINIMUM ULTIMATE COMPRESSIVE STRENGTH F'c AT 28 DAYS SHALL BE 8000 PSI.

REINFORCING STEEL:

DEFORMED BARS:

ASTM A615 GRADE 60 U.N.O. REINFORCING TO BE WELDED SHALL BE ASTM A706, GRADE 60, LOW ALLOY U.N.O.

WELDED WIRE FABRIC:

ASTM A82 AND ASTM A185. PROVIDE IN FLAT SHEETS.

WELDING:

UNLESS NOTED OTHERWISE, WELDING OF REINFORCING STEEL IS PROHIBITED. WHERE WELDING IS NOTED, WELD IN ACCORDANCE WITH AWS D1.4 USING E60 SERIES ELECTRODES. SUBMIT MILL TEST REPORTS ON THE REINFORCING TO BE WELDED AND THE PROPOSED WELDING PROCEDURE.

LAP SPLICES IN CONCRETE:

SEE TYPICAL DETAIL FOR LAP SPLICES IN CONCRETE BEAMS, WALLS, SLABS AND FOOTINGS. UNLESS NOTED OTHERWISE, LAP SPLICES IN CONCRETE BEAMS, WALLS, SLABS AND FOOTINGS SHALL BE CLASS "B" TENSION LAP SPLICES AND LAP SPLICES IN CONCRETE COLUMNS SHALL BE STANDARD COMPRESSION LAP SPLICES PER LATEST EDITION OF ACI 318. STAGGER ALTERNATE SPLICES A MINIMUM OF ONE LAP LENGTH. LAPS IN WELDED WIRE FABRIC SHALL BE MADE SO THAT THE OVERLAP MEASURED BETWEEN OUTERMOST CROSS WIRES OF EACH FABRIC SHEET IS NOT LESS THAN THE SPACING OF CROSS WIRES PLUS 2 INCHES. ALL SPLICE LOCATIONS SUBJECT TO APPROVAL.

PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT CORNERS AND INTERSECTIONS OF FOOTINGS AND WALLS. SPACING SHOWN FOR REINFORCING BARS ARE MAXIMUM ON CENTERS. ALL BARS PER CRSI SPECIFICATIONS AND HANDBOOK. DOWEL ALL VERTICAL REINFORCING TO THE FOUNDATION. CONCRETE COLUMN DOWEL EMBEDMENT SHALL BE A STANDARD COMPRESSION DOWEL EMBEDMENT LENGTH ACCORDING TO THE LATEST EDITION OF ACI 318. SECURELY TIE ALL BARS IN POSITION PRIOR TO PLACING CONCRETE. SUPPORT SLAB REINFORCEMENT WITH CONCRETE BLOCKS OR SUPPORT CHAIRS. REINFORCING SHALL NOT BE SUPPORTED BY STAKES DRIVEN INTO THE GROUND.

STRUCTURAL STEEL:

ROLLED SHAPES OTHER THAN WIDE-FLANGE SHAPES,	
ALL PLATES, BARS AND RODS	ASTM A36, Fy = 36 KSI
ALL WIDE-FLANGE SHAPES	ASTM A992, Fy = 50 KSI
HSS STEEL	ASTM A500 GR. B
PIPE STEEL	ASTM A53, Fy = 35 KSI
BOLTS	ASTM A307
ANCHOR BOLTS	ASTM A36 ROD STOCK OR F1554 GR. 36
DEFORMED BAR ANCHORS	ASTM A496, Fy = 70 KSI

ALL STEEL EXPOSED TO WEATHER TO BE HOT-DIPPED GALVANIZED U.N.O.

FABRICATION AND ERECTION:

LATEST AISC AND AWS CODES APPLY. FABRICATE AND ERECT IN ACCORDANCE WITH LATEST EDITION OF AISC "SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS". SPLICING OF STRUCTURAL MEMBERS IS NOT PERMITTED UNLESS NOTED ON THE DRAWINGS. ALL BEAMS SHALL BE ERECTED WITH THE NATURAL CAMBER UPWARDS.

ALL SURFACES FOR SLIP-CRITICAL BOLTED CONNECTIONS SHALL BE FREE OF PAINT, INCLUDING ANY INADVERTENT OVERSPRAY, IN AREAS CLOSER THAN ONE BOLT DIAMETER, BUT NOT LESS THAN ONE INCH, FROM THE EDGE OF ANY HOLE AND AT ALL AREAS WITHIN THE BOLT PATTERN.

WELDING:

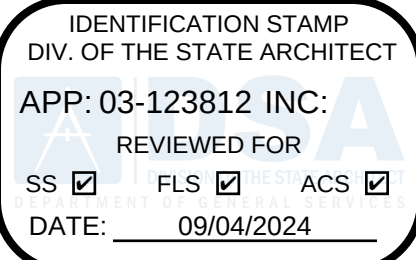
ALL WELDING SHALL BE BY CERTIFIED WELDERS HAVING CURRENT EXPERIENCE IN TYPE OF WELD SHOWN ON DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY. ALL WELDING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF AWS "CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION". USE E70 SERIES LOW HYDROGEN ELECTRODES. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON SHOP DRAWINGS. ALL COMPLETE PENETRATION WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT TESTING AGENCY. ALL DEFORMED BAR ANCHORS, HEADED STUDS AND THREADED STUDS SHALL BE END WELDED PER MANUFACTURER'S RECOMMENDATIONS.

BOLTS:

ALL BOLTS, ANCHOR BOLTS, EXPANSION BOLTS, ETC., SHALL BE INSTALLED WITH STEEL WASHERS, USE TYPE X BOLTS U.N.O. PER LATEST EDITION OF AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" AND SHALL BE TIGHTENED TO THE SNUG-TIGHT CONDITION AS DEFINED BY AISC UNLESS NOTED OTHERWISE.

SNUG-TIGHT IS DEFINED AS THE TIGHTNESS ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH. SNUG TIGHTENING SHALL PROGRESS SYSTEMATICALLY FROM THE MOST RIGID PART OF THE CONNECTION TO THE FREE EDGES, AND THEN THE BOLTS OF THE CONNECTION SHALL BE RETIGHTENED IN A SIMILAR SYSTEMATIC MANNER AS NECESSARY UNTIL ALL BOLTS ARE SIMULTANEOUSLY SNUG TIGHT AND THE CONNECTION IS FULLY COMPACTED.

DWG NUMBER	SHEET TITLE
S-0.1	GENERAL NOTES
S-0.2	GENERAL NOTES
S-1.1	TYPICAL DETAILS
S-1.2	DETAILS
S-2.1	FOUNDATION PLAN
S-2.2	FIRST FLOOR FRAMING PLAN
S-3.1	ENLARGED PLANS
S-4.1	ELEVATIONS



M&O - A/E SERVICES

CONSULTANT

STAMPS/SEALS



09/30/2025

DSA PHASE 1 BACK CHECK

SHEET TITLE:

GENERAL NOTES

PROJECT NO.: 230017

PROJECT ENGR:

DRAWN:

CHECKED:

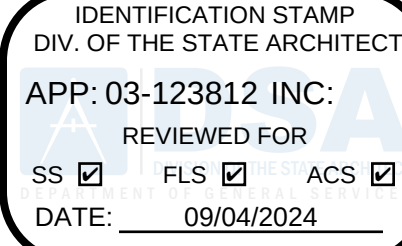
SHEET NUMBER

S-0.1

DATE:

SHEET 40

OF: 63



**LOS ANGELES UNIFIED
SCHOOL DISTRICT**

**M&O - A/E SERVICES
FACILITIES SERVICES DIVISION**

333 S. BEAUDRY AVENUE, 22ND FLOOR
LOS ANGELES, CALIFORNIA 90017
TEL: (213)241-3952

PROJECT TITLE AND SCHOOL LOCATION

**EVANS CAS -
ADA UPGRADES AND
BRIDGE REMOVAL**

717 North Figueroa Street
Los Angeles, California 90012

COLIN NUMBER: 10372448

COMMISSIONED ARCHITECT

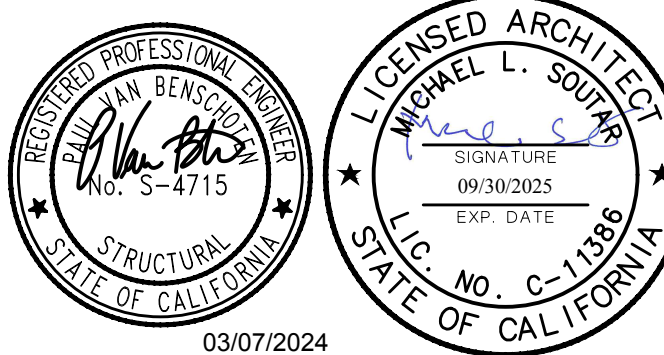
Architects McDonald, Soutar & Paz, Inc.

3575 LONG BEACH BOULEVARD
LONG BEACH CALIFORNIA 90807
TEL (562) 427-5007 FAX (562) 427-3007
ARCHITECTSMSP.COM

CONSULTANT



STAMPS/SEALS



03/07/2024

100% SUBMITTAL

SHEET TITLE

GENERAL NOTES

PROJECT NO.: 230017

PROJECT ENGR: JH / PVE

DRAWN: MI

CHECKED: JH / PVE

SHEET NUMBER

S-0.2

DATE: 02 2

02 2

41 OF: 63

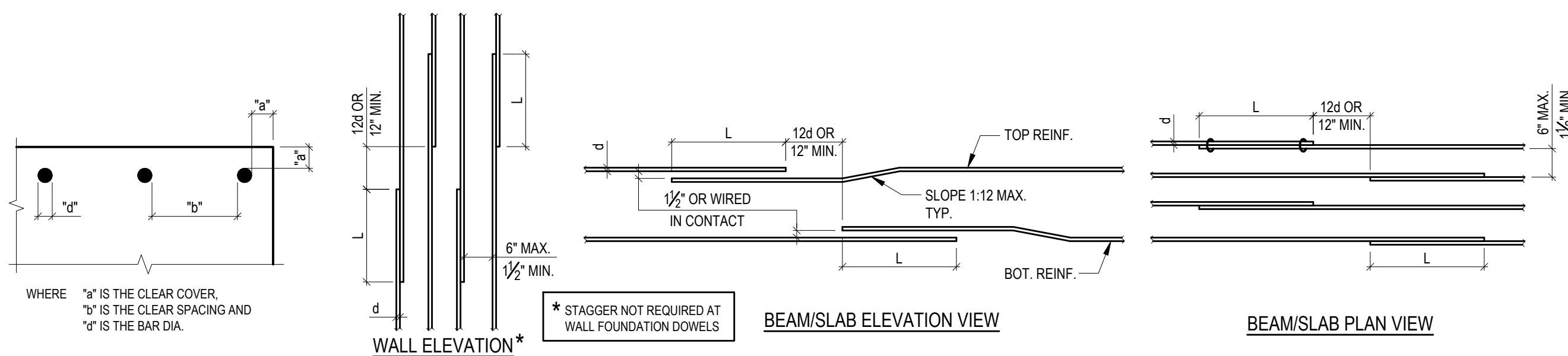
PRINTED DATE: March 7, 202

LAP SPLICE SCHEDULE "CLASS B" (STRAIGHT BARS)															
	CONCRETE STRENGTH F _c	BAR													
		#4		#5		#6		#7		#8		#9		#10	
		TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER
CLEAR COVER ≥ d AND CLEAR SPACING ≥ 2d	3000 PSI	37	29	47	36	56	43	81	63	93	72	105	81	118	91
	4000 PSI	33	25	41	31	49	37	71	54	81	62	91	70	102	79
	5000 PSI	29	23	36	28	43	34	63	49	72	56	81	63	92	70
OTHER CASES	3000 PSI	56	43	70	54	84	65	122	94	139	107	157	121	177	136
	4000 PSI	49	37	61	47	73	56	106	81	121	93	136	105	153	118
	5000 PSI	43	32	54	42	65	50	95	73	108	83	122	94	137	106

DEVELOPMENT LENGTH SCHEDULE "CLASS A" (STRAIGHT & HOOKED BARS)															
	CONCRETE STRENGTH F _c	BAR													
		#4		#5		#6		#7		#8		#9		#10	
		TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER
STRAIGHT CLEAR COVER ≥ d AND CLEAR SPACING ≥ 2d	3000 PSI	29	22	36	28	43	33	63	48	72	55	81	62	91	70
	4000 PSI	25	19	31	24	37	29	54	42	62	48	70	54	79	61
	5000 PSI	23	17	28	22	34	26	49	38	56	43	63	48	70	54
OTHER CASES	3000 PSI	43	33	54	42	65	50	94	72	107	83	121	93	136	105
	4000 PSI	37	29	47	36	56	43	81	63	93	72	105	81	118	91
	5000 PSI	34	26	42	32	50	39	73	56	83	64	94	72	106	81
HOOK STANDARD	3000 PSI		11		14		17		20		22		25		28
	4000 PSI		10		12		15		17		19		22		25
	5000 PSI		9		11		13		15		17		20		24

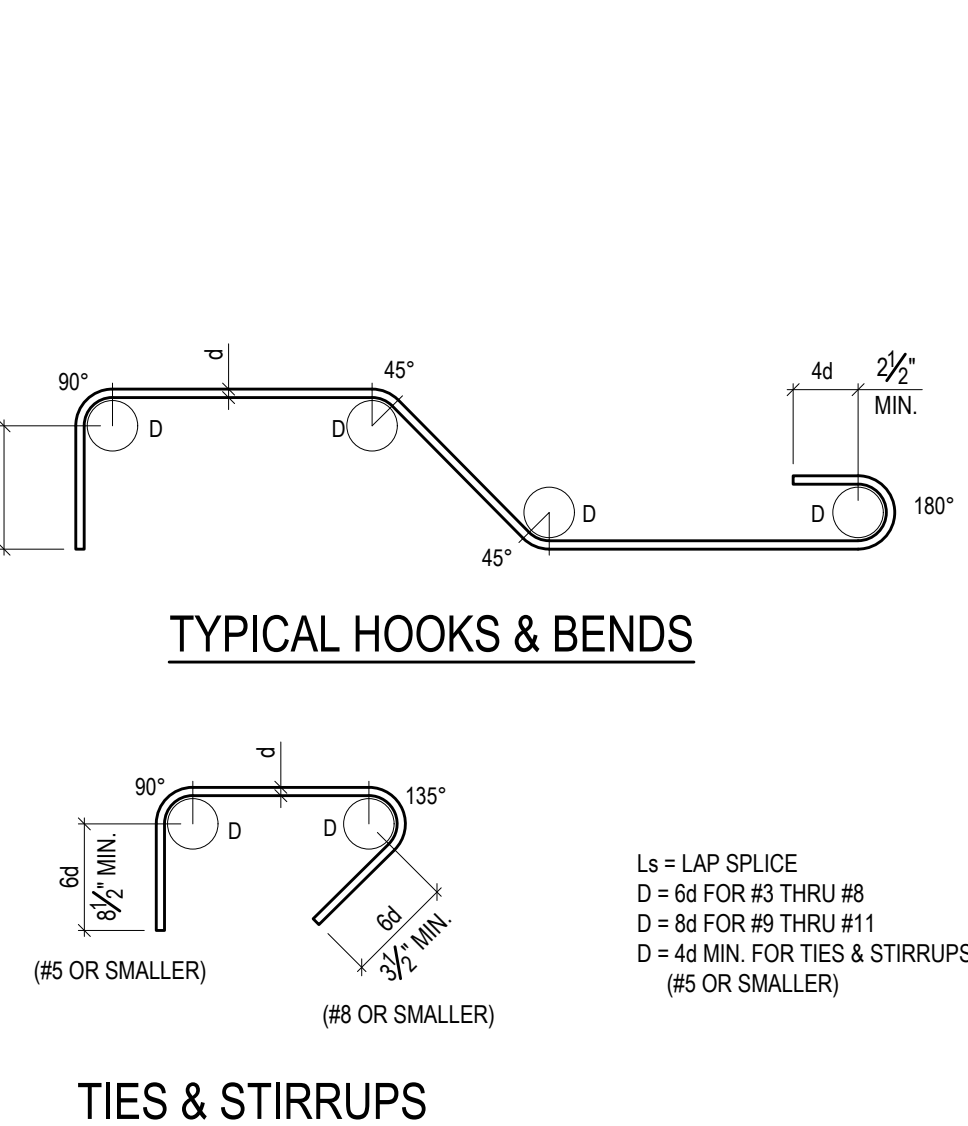
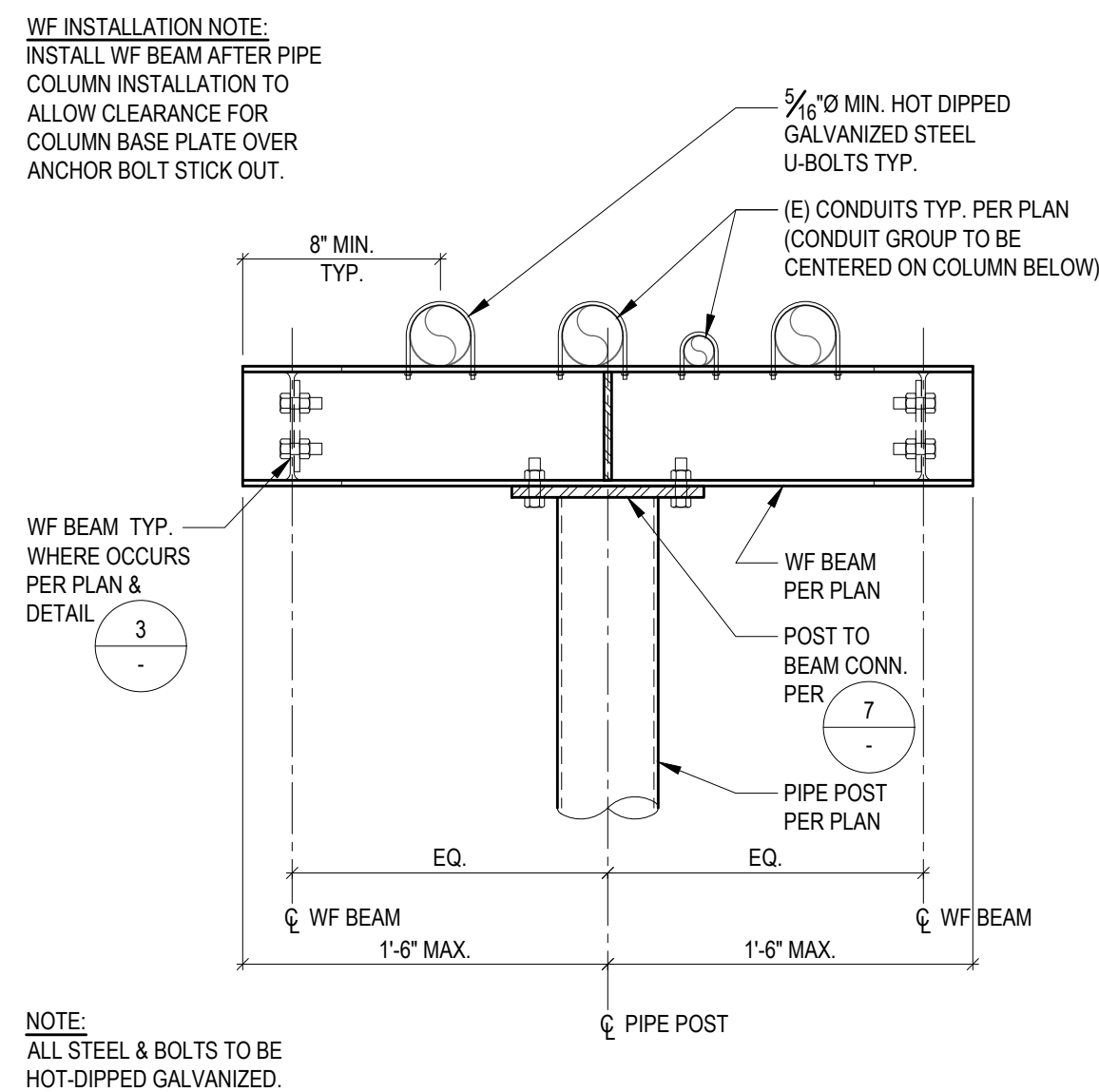
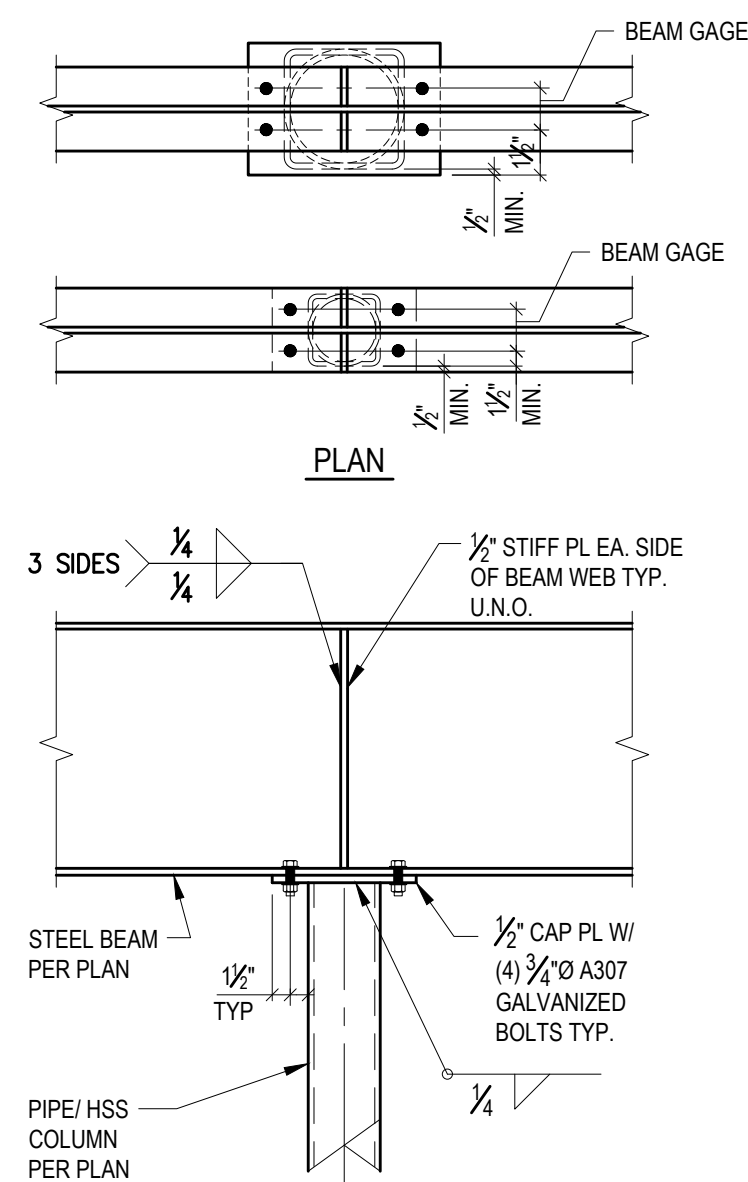
NOTE:
1. T=TOP BAR, L=LAP SPLICE LENGTH
2. "d" INDICATES BAR DIAMETER.
3. ALL DIMENSIONS SHOWN IN TABLE ABOVE ARE IN INCHES.

- NOTES:
- TABULATED VALUES AND NOTES ARE PER ACI318-19.
 - SPLICES SHALL BE CONTACT TENSION LAP SPLICES U.N.O.
 - LENGTHS SHOWN ARE FOR GRADE 60 UNCOATED BARS
 - ALL LAP SPLICES SHALL BE CLASS B, UNLESS CLASS A IS SPECIFICALLY APPROVED BY THE ENGINEER.
 - THE SCHEDULES SHOWN ON THIS DETAIL APPLY TO NORMAL WEIGHT CONCRETE.
 - FOR LIGHTWEIGHT CONCRETE, INCREASE LAP SPLICE LENGTH BY 33%.
 - ALL HORIZONTAL AND VERTICAL SPLICES SHALL BE STAGGERED AS SHOWN BELOW WHERE POSSIBLE U.N.O. WHERE SPLICES ARE NOT STAGGERED THE LAP SPLICE LENGTH SHALL BE INCREASED BY 30%.
 - THE SMALLER BAR LAP LENGTH SHALL BE USED WHEN SPLICING BARS OF DIFFERENT SIZES.
 - TOP BARS ARE HORIZONTAL BARS PLACED SO THAT MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE BAR.
 - LAP LENGTHS SPECIFICALLY DETAILED ON DRAWINGS SHALL GOVERN IN LIEU OF THE LAP LENGTHS SCHEDULED. OTHERWISE, ALL DETAILING AND PLACEMENT OF REINFORCING SHALL COMPLY WITH THE LAP SPLICE SCHEDULES AND DETAILS.
 - BUNDLED BAR SPLICES:
 - INDIVIDUAL BAR SPLICES WITHIN THE BUNDLE SHALL NOT OVERLAP EACH OTHER.
 - INCREASE LAP LENGTH 20% AT THREE BAR BUNDLES.
 - INCREASE LAP LENGTH 33% AT FOUR BAR BUNDLES.
 - FOR #14 AND #18 BARS, USE MECHANICAL SPLICE IN ACCORDANCE WITH ACI-318 REQUIREMENTS.
 - EPOXY COATED BAR DEVELOPMENT & SPLICES:
 - HOOKS: INCREASE DEVELOPMENT & LAP LENGTH 20%.
 - CONC. STRAIGHT BARS:
 - WHEN CONCRETE COVER IS LESS THAN 3d OR CLEAR SPACING IS LESS THAN 6d, INCREASE DEVELOPMENT & LAP LENGTH 50%.
 - FOR OTHER CASES, INCREASE LAP LENGTH 20%.
 - HOOK PORTION OF SCHEDULE ASSUMES A STANDARD 90° HOOK.
 - SEISMIC CONDITIONS:
 - SEISMIC DIAPHRAGM AND COLLECTOR BARS REQUIRE NO INCREASE TO HOOK OR LAP SPLICE LENGTHS.
 - VERTICAL BARS OF SPECIAL REINFORCED CONCRETE WALLS SHALL HAVE A BOTTOM BAR LAP SPLICE INCREASED BY 25%.
 - DEVELOPMENT LENGTHS AND LAP SPLICES FOR CONCRETE SPECIAL MOMENT RESISTING FRAMES ARE NOT COVERED BY THIS DETAIL AND MUST MEET ALL ACI 318 CHAPTER 18 REQUIREMENTS.

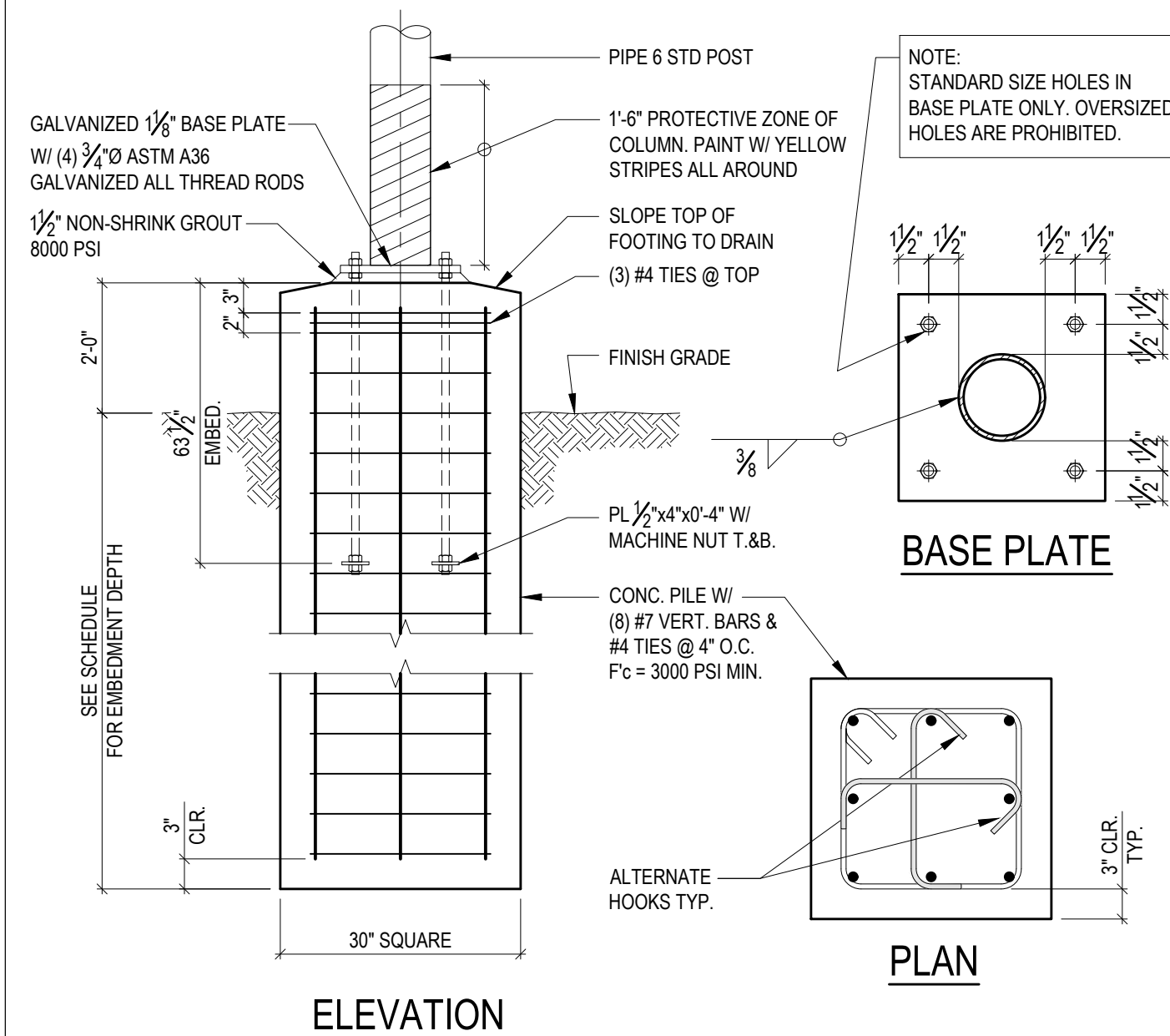


5 TYPICAL NORMAL WEIGHT CONCRETE REINFORCING LAP SPLICE SCHEDULE

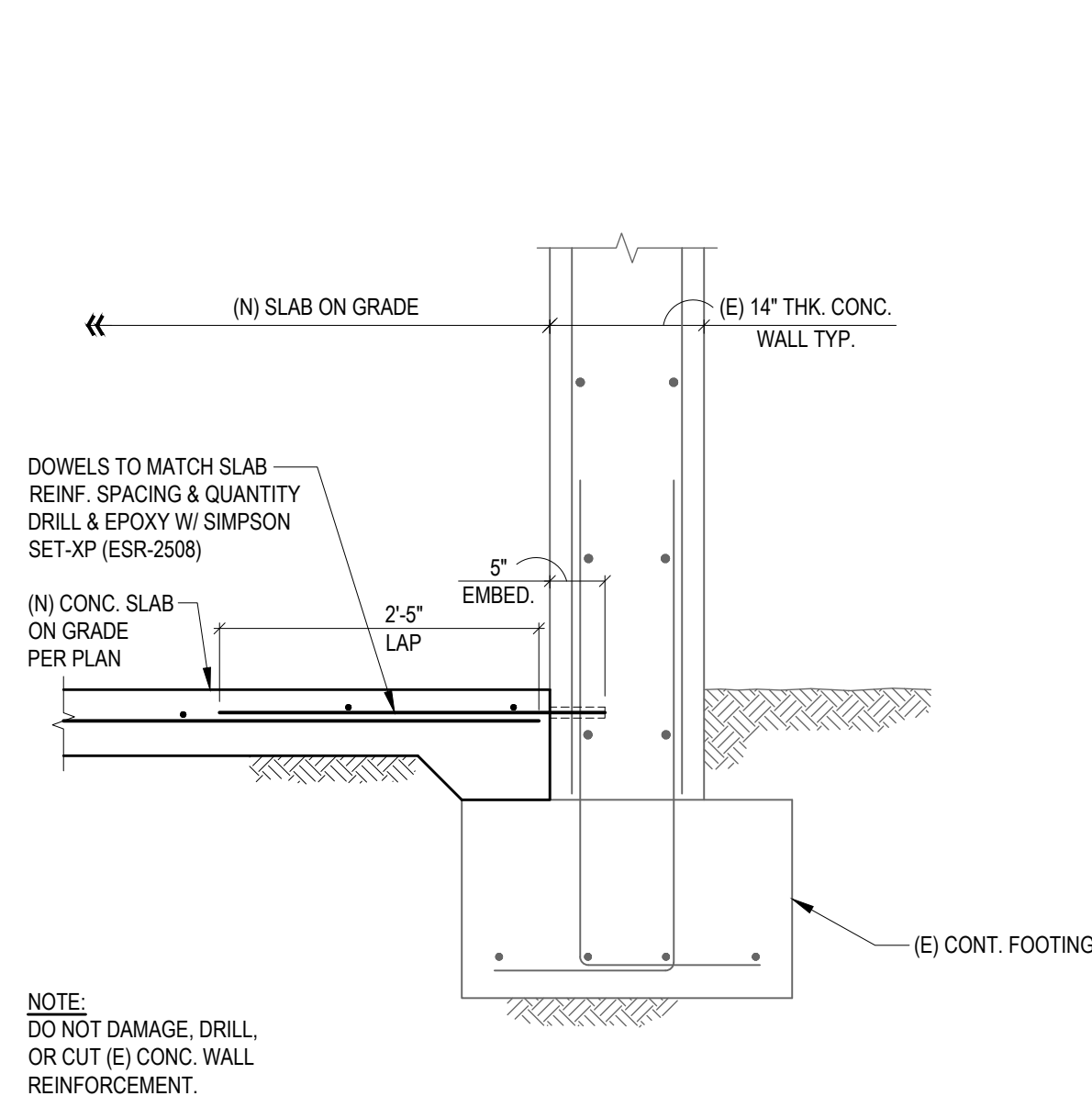
7 PIPE/ HSS COLUMN CONNECTION AT BEAM



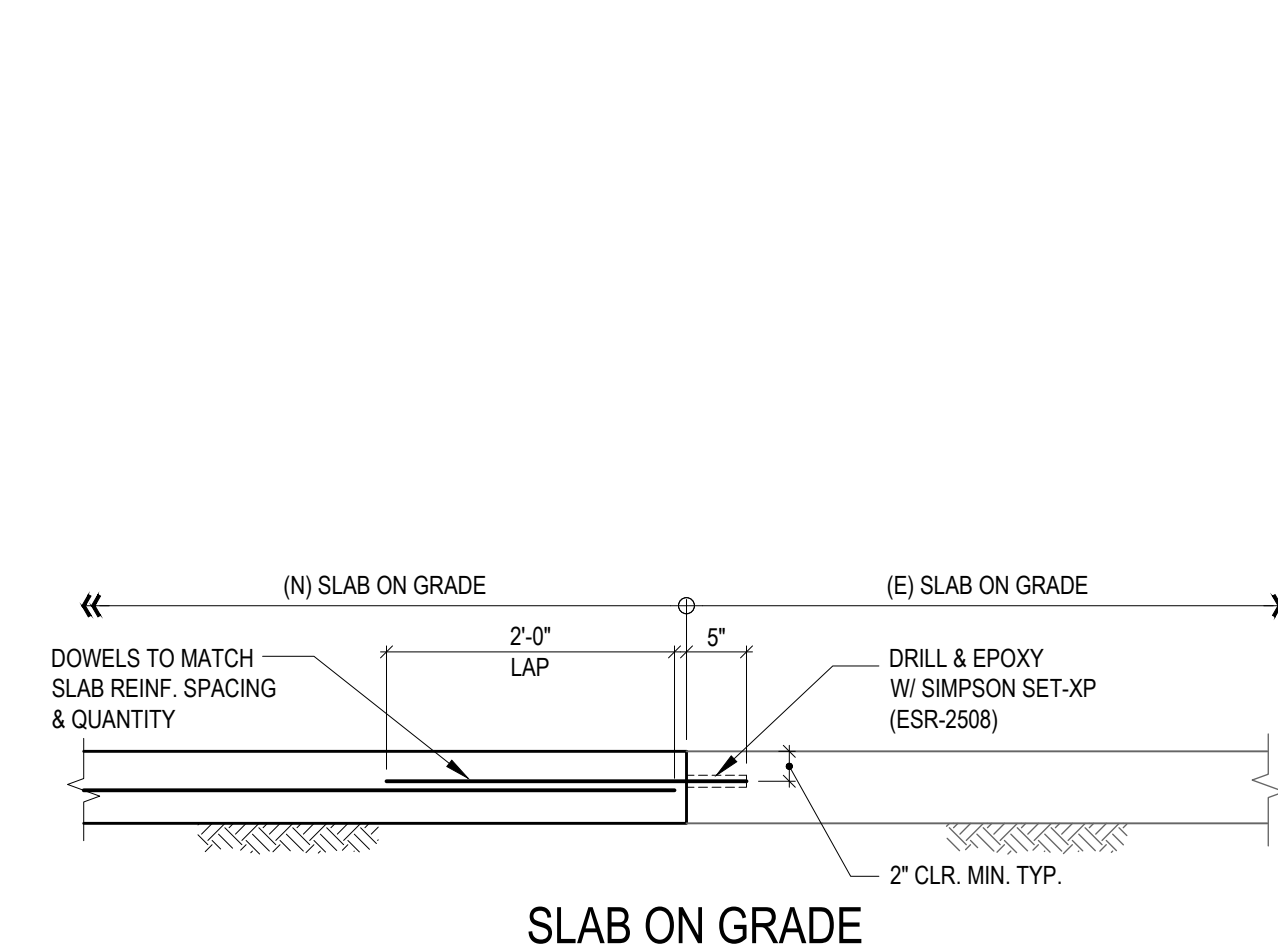
9 REINFORCING BAR DETAILS



10 PIPE POST FOOTING



11 (N) SLAB DOWEL TO (E) CONC. WALL SECTION



12 EXISTING TO NEW SLAB ON GRADE

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PROJECT TITLE AND SCHOOL LOCATION

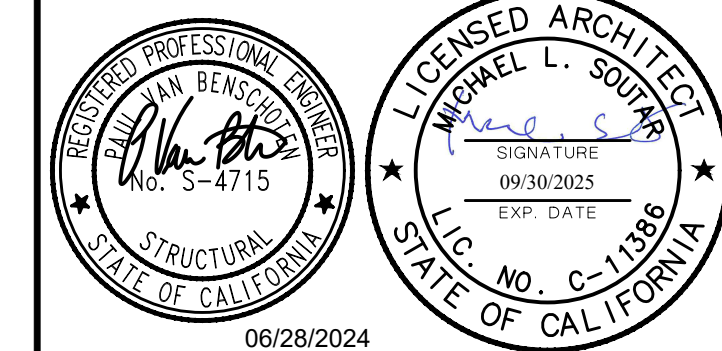
EVANS CAS -
ADA UPGRADES AND
BRIDGE REMOVAL
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COLIN NUMBER: 10372448

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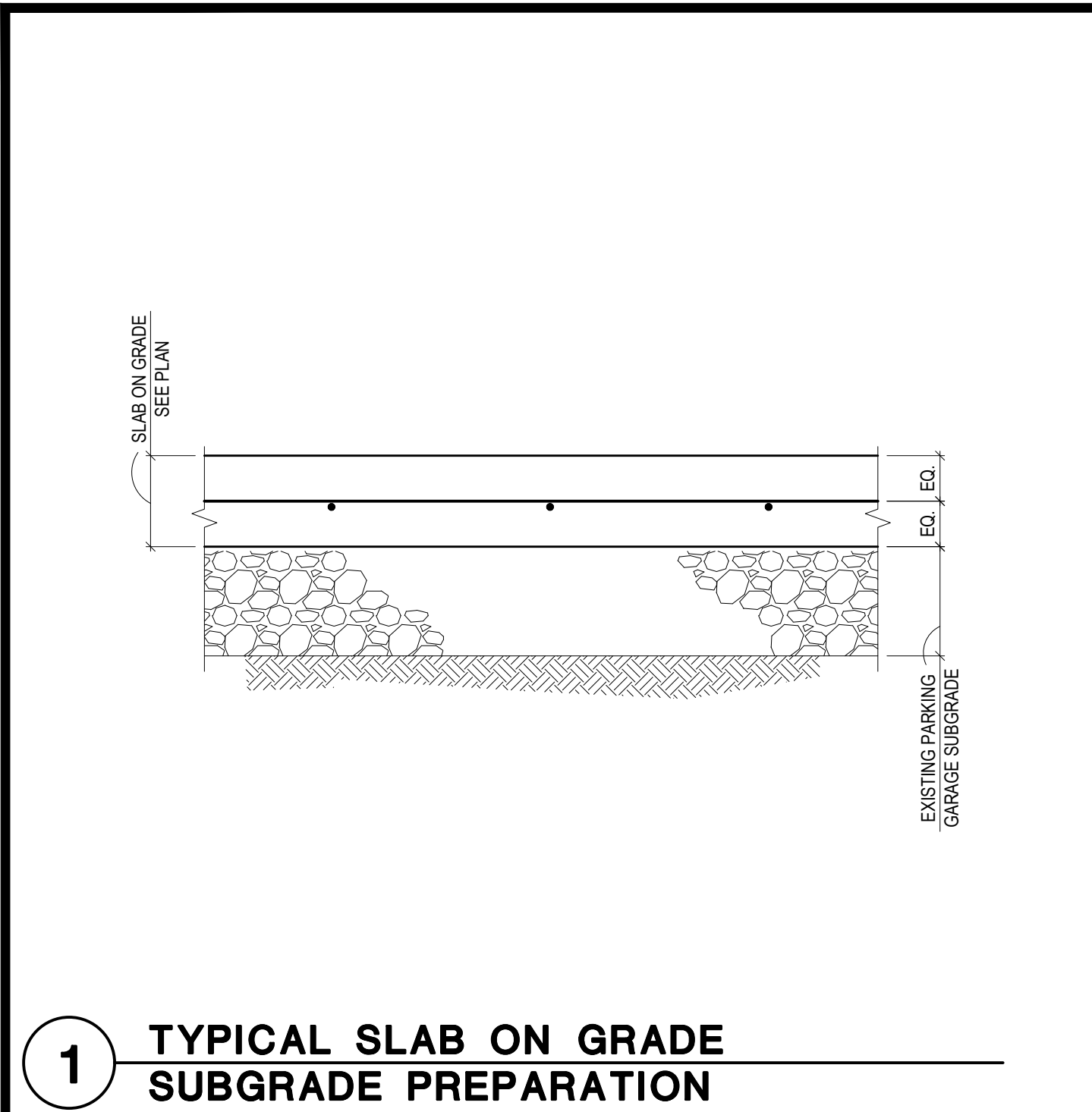
TYPICAL DETAILS

PROJECT NO.: 230017 PROJECT ENGR: JH / PVB
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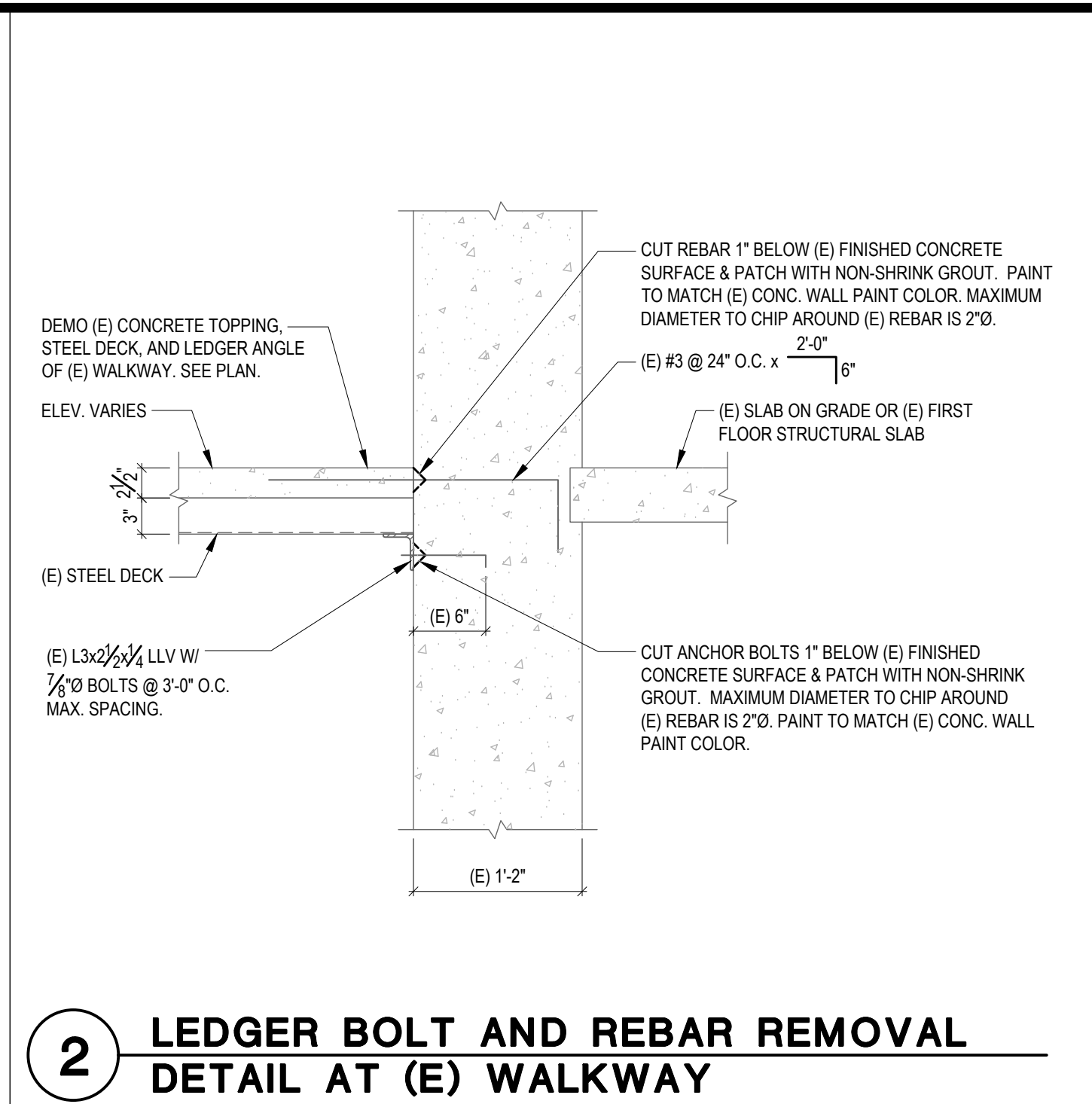
S-1.1

DATE: 02.27.24 SHEET: 42 OF: 63

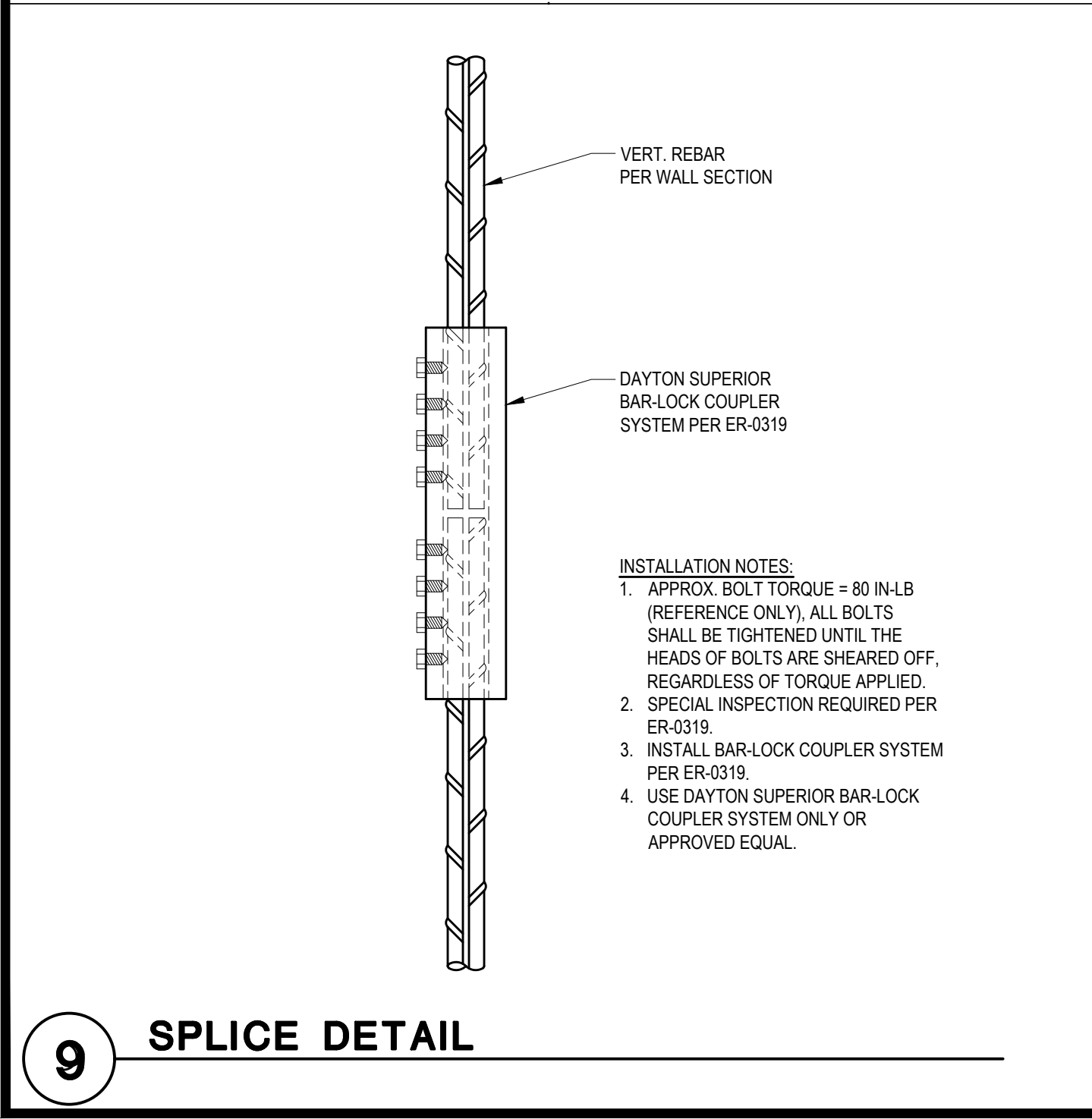
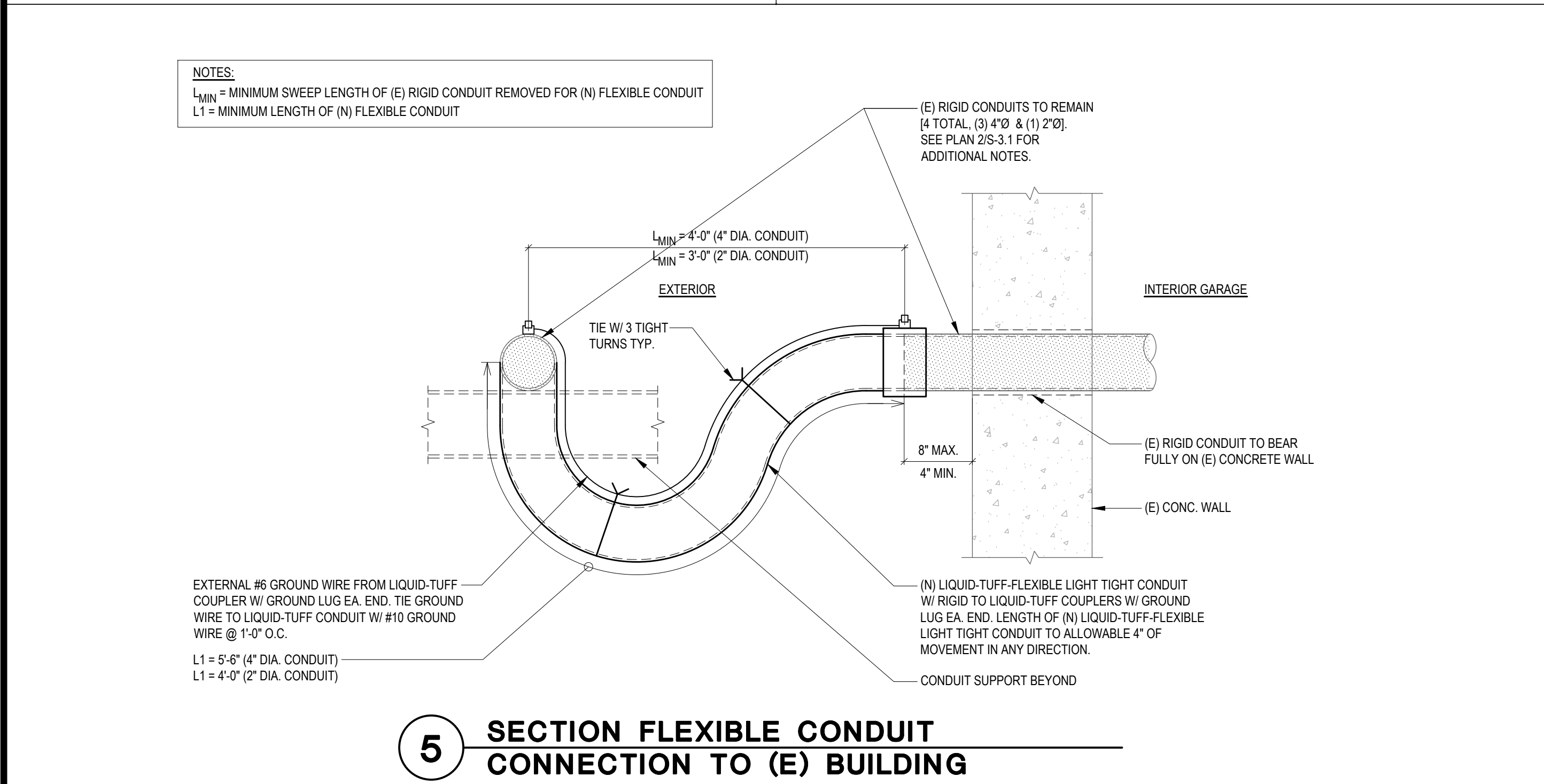
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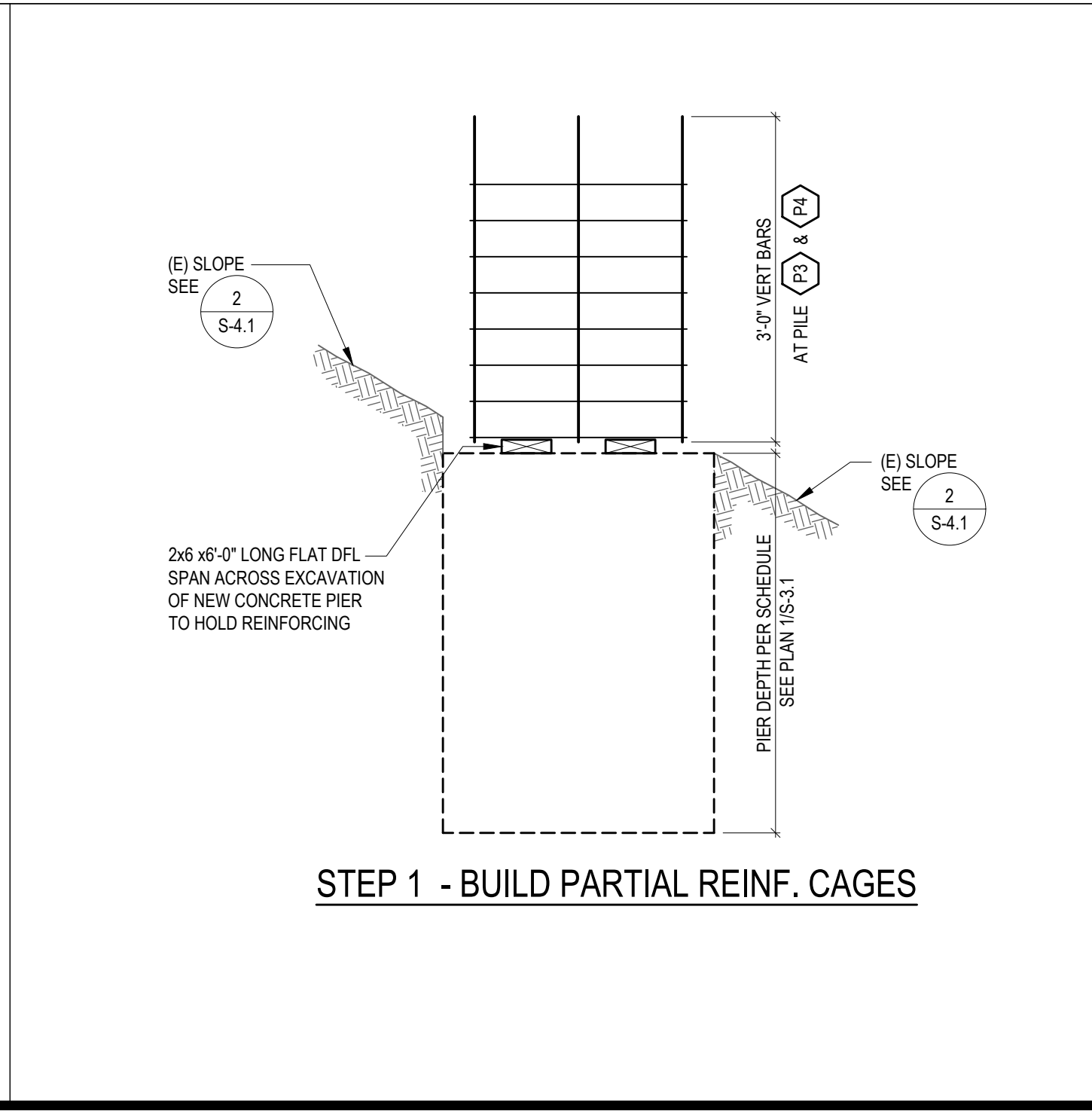
1 TYPICAL SLAB ON GRADE SUBGRADE PREPARATION



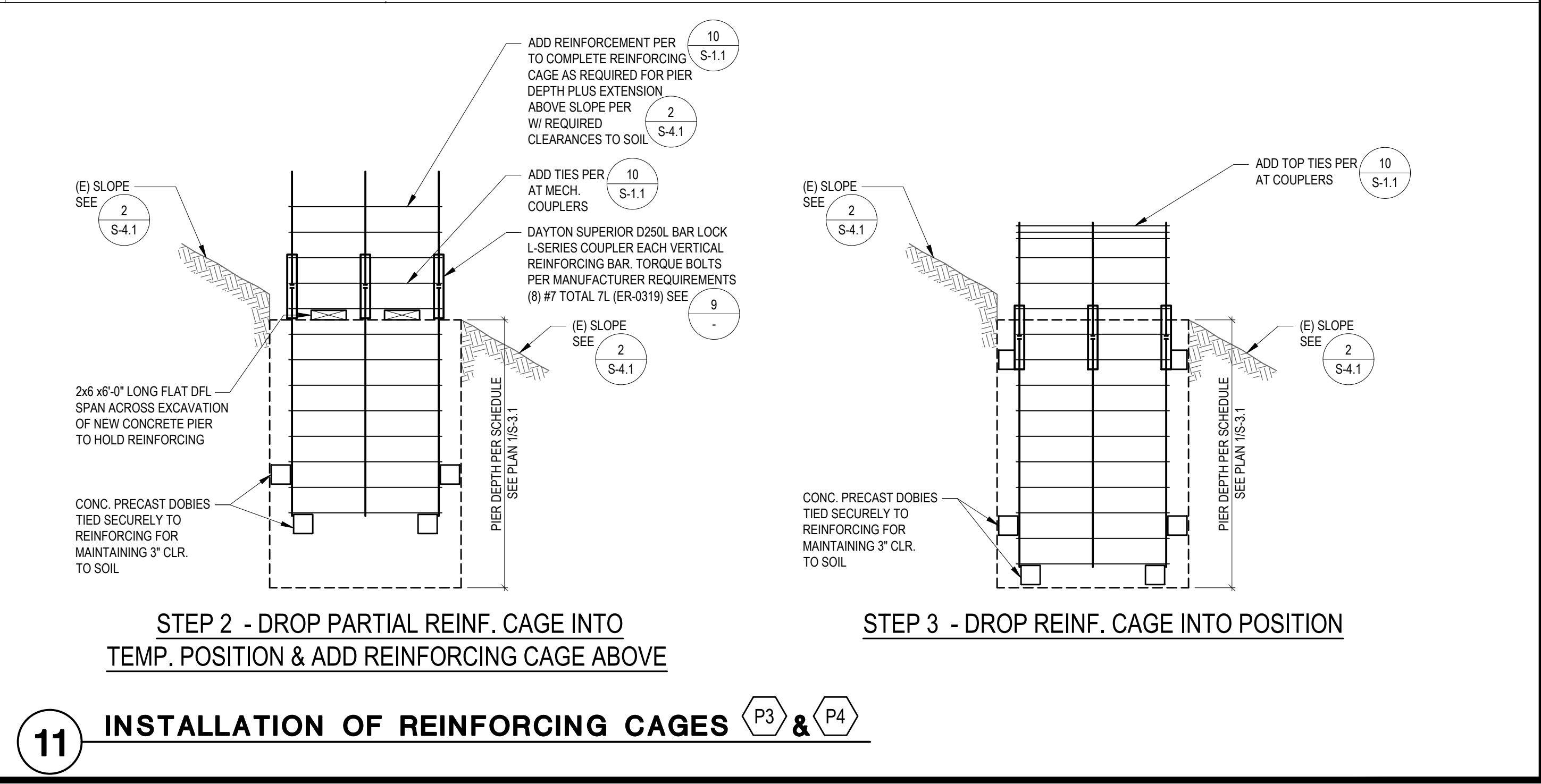
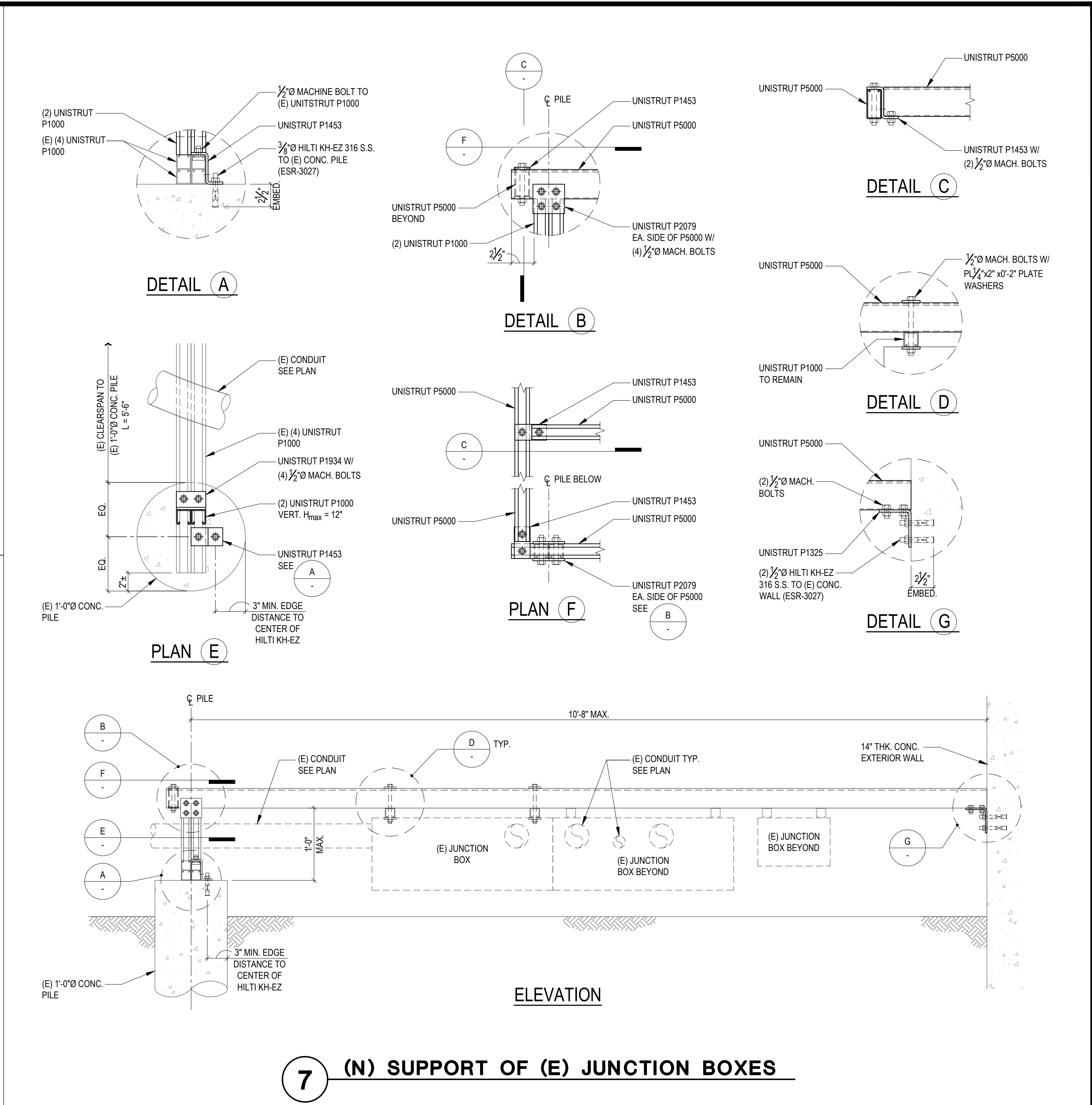
2 LEDGER BOLT AND REBAR REMOVAL DETAIL AT (E) WALKWAY



9 SPLICE DETAIL



10 INSTALLATION OF REINFORCING CAGES



11 INSTALLATION OF REINFORCING CAGES

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SHEET TITLE:

DETAILS

PROJECT NO.: 230017 PROJECT ENGR: JH / PVB
DRAWN: MBH CHECKED: JH / PVB
SHEET NUMBER

S-1.2

DATE: 07.03.24 SHEET: 43 OF: 63
PRINTED DATE: 07/03/2024



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PROJECT TITLE AND SCHOOL LOCATION

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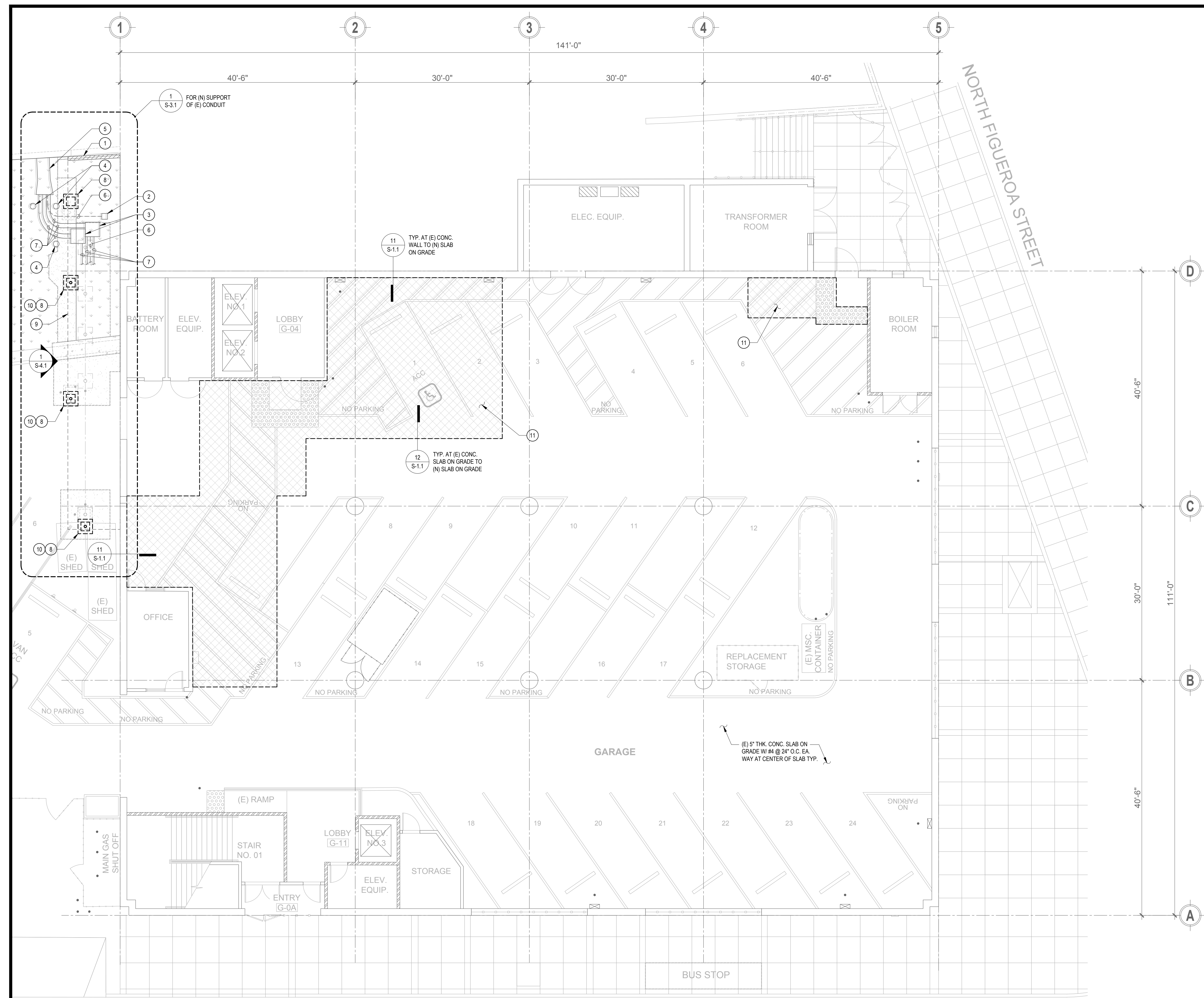
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FOUNDATION PLAN

PROJECT NO.: 230017 PROJECT ENGR: JH / PVB
DRAWN: MBH CHECKED: JH / PVB
SHEET NUMBER
S-2.1
DATE: 02.27.24 SHEET: 44 OF: 63
PRINTED DATE: March 7, 2024



- KEY NOTES:**
- (E) RETAINING WALL BELOW WALKWAY DECK.
 - (E) 12"x12"x8" JUNCTION BOX TO REMAIN.
 - (E) 30"x30"x12" JUNCTION BOX TO REMAIN.
 - (E) 12"Ø CONC. PILES TO REMAIN.
 - (E) CONC. CONDUIT ENCASEMENT TO REMAIN.
 - (E) 2"Ø CONDUITS TO REMAIN.
 - (E) 4"Ø CONDUITS TO REMAIN.
 - (E) COLUMNS AND FOOTING TO BE DEMOLISHED AFTER REMOVING FRAMING OF THE BALCONY ABOVE.
 - LINE OF (E) BALCONY ABOVE TO BE DEMOLISHED.
 - (E) 3"Ø PIPE COLUMN, 18"x18" PEDESTAL & 2'-6" SQUARE x1'-0" DEEP FOOTING. COLUMN & PEDESTAL HEIGHTS VARY. SEE ELEVATIONS ON 1/5-4.1 FOR EXISTING HEIGHTS.
 - SAW CUT AND REMOVE (E) SLAB ON GRADE AT HATCHED AREA SHOWN AS: [Hatched Area Symbol].
REPLACE W/ (N) 5" THK CONC. SLAB ON GRADE W/ #4 @ 24" O.C. EA. WAY CENTERED ON SLAB.
SEE CIVIL DRAWING SHEET C3.0 FOR FINISHED SLAB ELEVATIONS.

- NOTES:**
- FIELD VERIFY EXISTING CONDITIONS. NOTIFY ARCHITECT IF EXISTING CONDITION IS DIFFERENT THAN WHAT IS SHOWN IN THIS DRAWING.
 - VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
 - SHORE & PROTECT (E) CONDUIT & JUNCTION BOXES TO REMAIN IN PLACE. DO NOT DAMAGE (E) CONDUITS & JUNCTION BOXES.



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03/07/2024

FIRST FLOOR FRAMING PLAN

DATE	02.27.24	OFFICE	45	ST	63
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PRINTED DATE: March 7, 2023

02 27 24 SHEET: 45 OF: 63

63

PRINTED DATE: March 7, 2023



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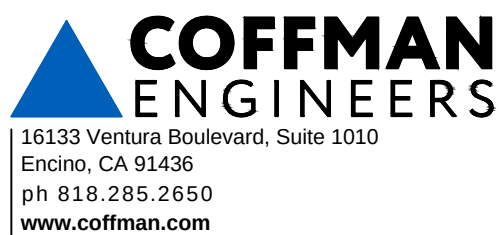
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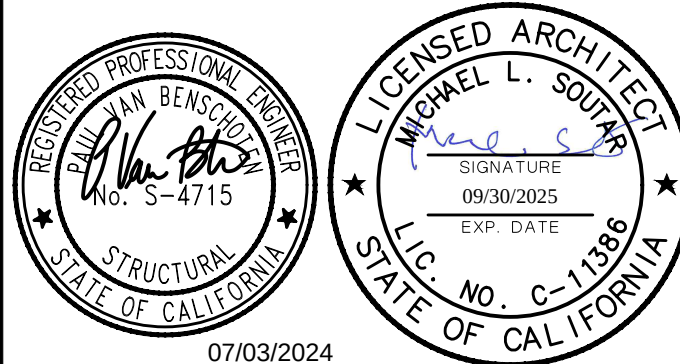
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SHEET TITLE:

ENLARGED PLANS

PROJECT NO.: 230017 PROJECT ENGR: JH / PVB

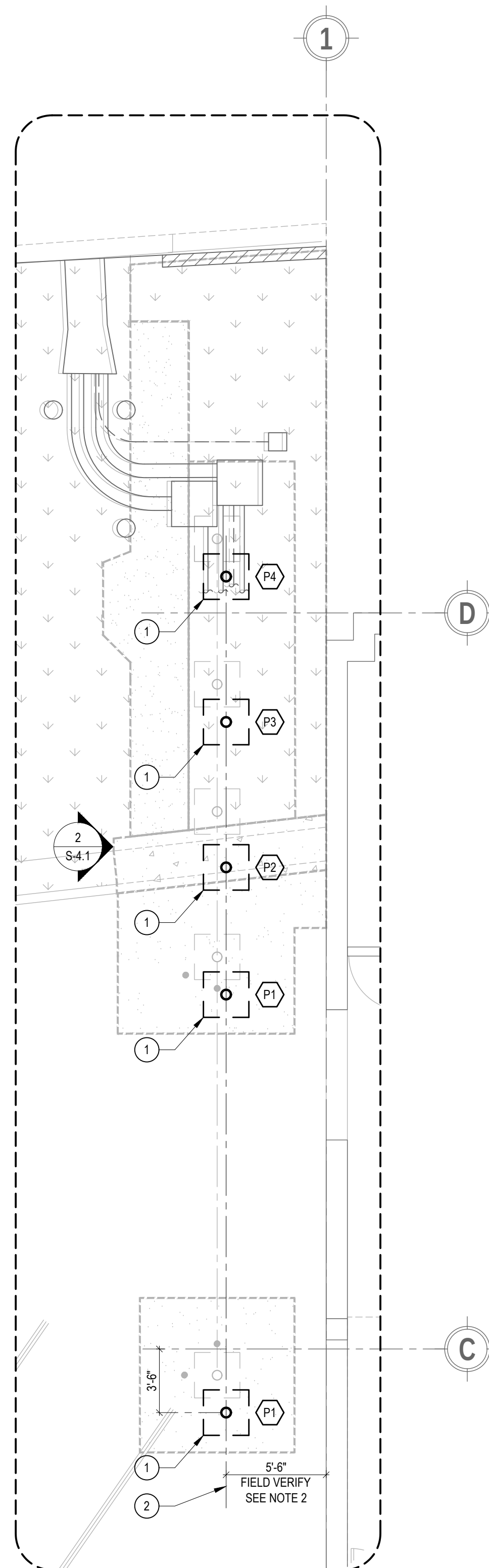
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SHEET NUMBER

S-3.1

DATE: 07.03.24 SHEET: 46 OF: 63

PRINTED DATE: 07/03/2024



PILE DEPTH SCHEDULE		
MARK	DEPTH []	DETAIL REFERENCE
P1	5'-6"	10S-1.1
P2	4'-6"	10S-1.1
P3	3'-6"	10S-1.1 & 11S-1.2
P4	3'-6"	10S-1.1 & 11S-1.2

BELOW LOWEST ADJACENT GRADE

NOTES:

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- VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- SHORE & PROTECT (E) CONDUIT & UNCTION BOXES TO REMAIN IN PLACE. TEMPORARY SHORING IS MEANS AND METHODS. CONTRACTOR TO PROVIDE TEMPORARY SHORING OF EXISTING CONDUITS AND UNCTION BOXES AS REQUIRED TO INSTALL NEW CONDUIT AND UNCTION BOX SUPPORTS. DO NOT DAMAGE (E) CONDUITS & UNCTION BOXES.
- SEE ELEVATIONS ON SHEET S4.1 FOR LAYOUT DIMENSIONS OF COLUMNS AND PILES.

KEY NOTES:

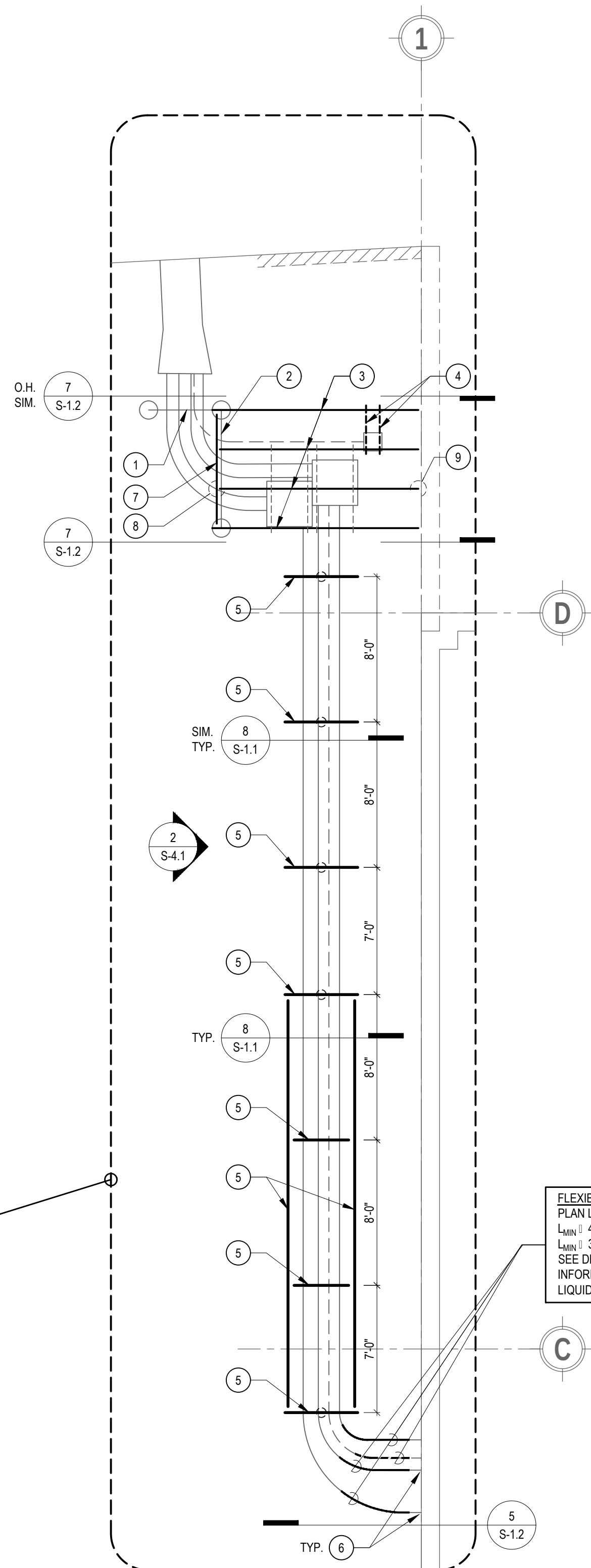
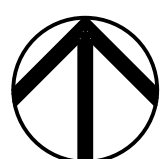
- (N) PIPE 6 STD. POST AND 2'-6" SQ. CONC. PILE. SEE DETAIL 10S-1.1

- CENTERLINE OF PIPE COLUMNS & PILES TO BE CENTERED ON CONDUIT GROUP AS SHOWN ON

1 ENLARGED FOUNDATION PLAN

SCALE: 3/16" = 1'-0"

NORTH



EXTENT OF TEMPORARY SHORING OF (E) CONDUITS AND UNCTION BOXES. SEE NOTE 13 BELOW.

FLEXIBLE LIQUID-TUFF CONDUIT
PLAN LENGTH:
L_{MIN} = 4'-0" (4" DIA. CONDUIT)
L_{MIN} = 3'-0" (2" DIA. CONDUIT)
SEE DETAIL S-1.2 FOR ADDITIONAL INFORMATION FOR (N) FLEXIBLE LIQUID-TUFF CONDUIT.

NOTES:

- FIELD VERIFY EXISTING CONDITIONS. NOTIFY ARCHITECT IF EXISTING CONDITION IS DIFFERENT THAN WHAT IS SHOWN IN THIS DRAWING.
- VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- SHORE & PROTECT (E) CONDUIT & UNCTION BOXES TO REMAIN IN PLACE. TEMPORARY SHORING IS MEANS AND METHODS. CONTRACTOR TO PROVIDE TEMPORARY SHORING OF EXISTING CONDUITS AND UNCTION BOXES AS REQUIRED TO INSTALL NEW CONDUIT AND UNCTION BOX SUPPORTS. DO NOT DAMAGE (E) CONDUITS & UNCTION BOXES.

KEY NOTES:

- (E) EIGHT (8) UNISTRUT P1000 (2 - P1000 WIDE x 4 - P1000 HIGH) TO REMAIN.
- (E) FOUR (4) UNISTRUT P1000 (2 - P1000 WIDE x 2 - P1000 HIGH) TO REMAIN.
- (N) UNISTRUT P5000 FROM (E) CONC. WALL TO (E) 12" PILES TO SUPPORT (E) UNCTION BOXES.
- (N) UNISTRUT P1000 (TYP. 2 PLACES).
- (N) W8x24 STEEL MEMBERS ABOVE GARAGE DRIVEWAY OPENING TO SUPPORT (E) CONDUITS.

- FOUR (4) EXISTING CONDUITS ENTER THE EXISTING CONCRETE SHEARWALL BUILDING. SHALL HAVE WATERPROOF FLEXIBLE CONDUIT ALLOWING 4" OF MOVEMENT IN ANY DIRECTION. SEE DETAIL S-1.2 & ELECTRICAL NOTES ON E-2.

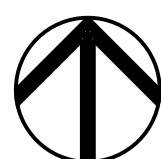
- (N) UNISTRUT P5000.
- (N) UNISTRUT P5000 TO P5000 CONN. (TYP. 2 PLACES) SEE DETAIL 7C S-1.2

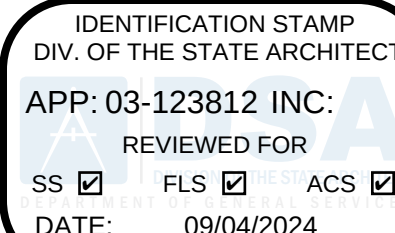
- (N) UNISTRUT P5000 TO (E) CONC. WALL. (TYP. 4 PLACES) SEE DETAIL 7G S-1.2

**2 ENLARGED CONDUIT
SUPPORT FRAMING PLAN**

SCALE: 3/16" = 1'-0"

NORTH





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PROJECT TITLE AND SCHOOL LOCATION

**EVANS CAS -
ADA UPGRADES AND
BRIDGE REMOVAL**

COLIN NUMBER: 103724

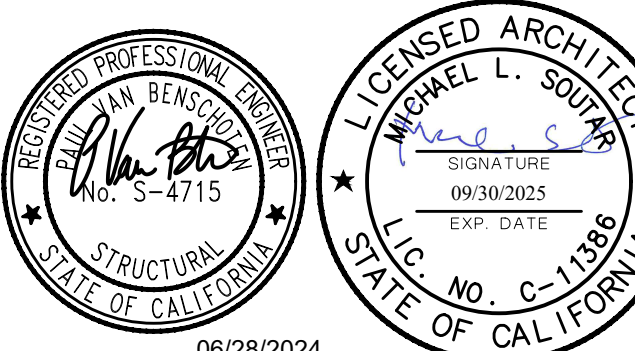
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ELEVATION

PROJECT NO.: 23

PROJECT ENGR: JH / P

DRAWN:

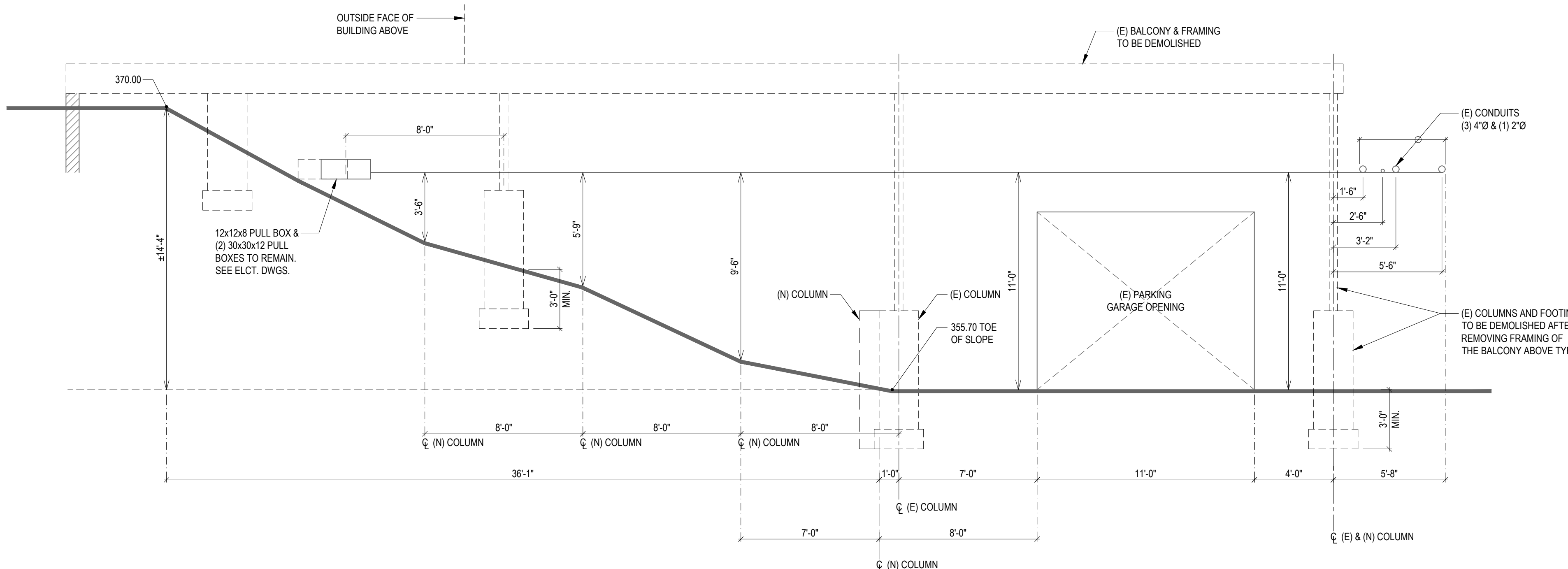
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S-4.

DATE: 03

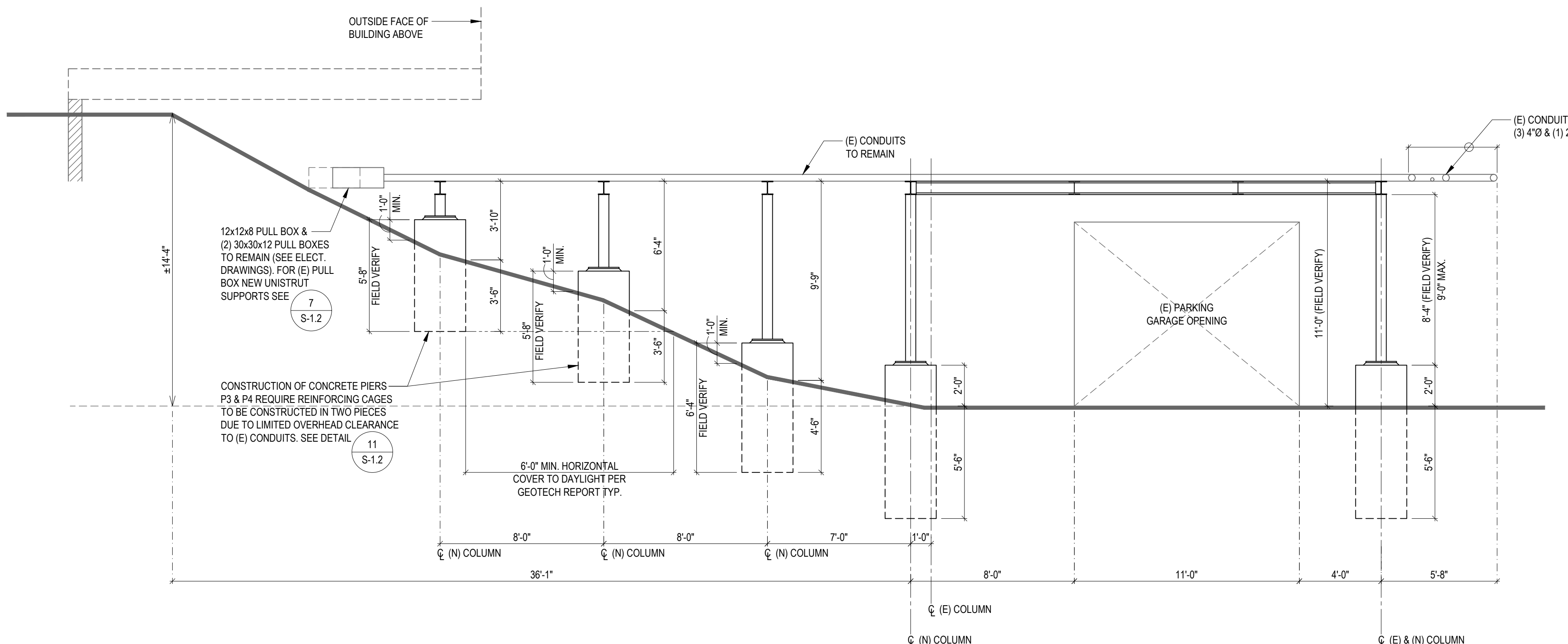
SHEET: 47 OF:



1. FIELD VERIFY EXISTING CONDITIONS

1. FIELD VERIFY EXISTING CONDITIONS

1 EXISTING WEST ELEVATION OF GARAGE
NOT TO SCALE



1. FIELD VERIFY EXISTING CONDITIONS

1. FIELD VERIFY EXISTING CONDITIONS

2 CONDUIT SUPPORT ELEVATION
- NOT TO SCALE